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BD

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DESIGN DISCHARGE	1,500 C.F.S.
FREQUENCY OF DESIGN FLOOD	25 YR.
DESIGN HIGH WATER ELEVATION	784.7
DRAINAGE AREA	5.3 SQ.MI.
BASE DISCHARGE (Q100)	2,100 C.F.S.
BASE HIGH WATER ELEVATION	786.93





	TOTAL BILL OF MATERIALS														
	REMOVAL OF EXISTING STRUCTURE	PILE EXCAVATION IN SOIL	PILE EXCAVATION NOT IN SOIL	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP 12 STEEL	2 X 53 . PILES	ANODIZED ONE BAR METAL RAIL	1"-0" x 1'-9 ¹ /2" CONCRETE PARAPET	RIP RAP CLASS II (2'-O" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3 P CON
	LUMP SUM	LIN.FT.	LIN.FT.	LUMP SUM	CU.YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	N
SUPERSTRUCTURE	LUMP SUM					LUMP SUM				125	140				9
END BENT NO.1		20	30		12.5		1,835	5	75.0						
END BENT NO.2		15	35		12.5		1,835	5	75.0			100	110		
TOTAL	LUMP SUM	35	65	LUMP SUM	25.0	LUMP SUM	3,670	10	150.0	125	140	100	110	LUMP SUM	9



LOCATION SKETCH



'-0" REST NCRE SL	X 2'-0" TRESSED TE CORED .ABS						
0.	LIN.FT.						
	630.0				RL)-5111/	R
		PR	UJECI	NU		JIIIA	<u> </u>
			Y	ADKIN			ΓY
	630.0	C 7		ς Τ Λ	12+20		
		51	A HON:	JIA.	12+20	.00 -L	
			SHEET 2 (DF 3			
		DWG. NO. 2 DWG. NO. 2 DWG. NO. 2 DWG. NO. 2 DWG. NO. 2 DWG. NO. 2	DEPA FOR ON SR 1	STA RTMENT GENEF BRIDGE SR 154 E DEA 538 (SH	TE OF NORTH CA OF TRAN RALEIGH COVER COVER 16 (SHOA BETWEEN DEND ADY GR(ROLINA NSPORTA WING HALL CR ALS ROA I AND OVE CH.	FION EEK D) RD.)
		KLAPPEr C		REVIS	SIONS	0.175	SHEET NO.
RIVE S	UITE 350	8/27/2015	NU. BY: 1	DATE:	NO. BY:	DATE:	J Z TOTAL
MBER:	27609-3960 F-0112	· · ·	2		4		17

NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN @ 36'-7" WITH AN ASPHALT WEARING SURFACE OVER A TIMBER FLOOR ON I-BEAM SUPERSTRUCTURE AND A CLEAR ROADWAY WIDTH OF 10.8' ON A SUBSTRUCTURE CONSISTING OF TIMBER CAPS/TIMBER POST & SILLS AND LOCATED AT THE PROPOSED STRUCTURE LOCATION SHALL BE REMOVED SEE SPECIAL PROVISION FOR "REMOVAL OF EXISTING STRUCTURE AT STA.12+20.60 -L-''.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF 25 FEET EACH SIDE OF CENTERLINE ROADWAY AT END BENT NO.1 AND END BENT NO. 2. EXISTING ROCK OUTCROP AT END BENT NO. 1 SHALL BE EXCAVATED TO THE ELEVATION SHOWN AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES".

M:\projec	
default 3/27/2015	DRAWN B CHECKED DESIGN

) V				
\geq	DRAWN BY :F.D. WEEDEN	DATE	:	AUG.2015
J	CHECKED BY : _ B.D. KLAPPENBACH	DATE	:	AUG. 2015
$\hat{\mathbf{o}}$	DESTGN ENGINEER OF RECORD : B.D. KLAPPENBACH	DATE		AUG. 2015

FOUNDATION NOTES:

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR ``REMOVAL OF EXISTING STRUCTURE AT STATION 12+20.60 -L-''.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR CONSTRUCTION STAGING, SEE TRAFFIC CONTROL PLANS.

FOR ANODIZED ONE BAR METAL RAIL, SEE SPECIAL PROVISIONS.

FACTORED RESISTANCE OF 95 TONS PER PILE.

DRIVING RESISTANCE OF 160 TONS PER PILE.

FIRST.

FIRST.



FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

- PILES AT END BENT NO.1 AND END BENT NO.2 ARE DESIGNED FOR A
- DRIVE PILES AT END BENT NO.1 AND END BENT NO.2 TO A REQUIRED
- PILE EXCAVATION IS REQUIRED TO INSTALL PILES AT END BENT NO.1. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 771 FEET. OR MINIMUM PENETRATION INTO ROCK OF 5 FEET, WHICHEVER OCCURS
- FOR PILE EXCAVATION. SEE SECTION 450 OF THE STANDARD SPECIFICATIONS
- PILE EXCAVATION IS REQUIRED TO INSTALL PILES AT END BENT NO.2. EXCAVATE HOLES AT PILE LOCATIONS TO ELEVATION 772 FEET. OR MINIMUM PENETRATION INTO ROCK OF 5 FEET. WHICHEVER OCCURS
- FOR PILE EXCAVATION. SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- CONCRETE OR GROUT IS REQUIRED TO FILL HOLES AT END BENT NO.1 AND NO.2.

	PR	OJECT	NO	BD)-5111A	B
		Y	ADKIN			ΓY
	ST	ATION:	STA.	12+20	.60 -L	-
		SHEET 3 (DF 3			
	DWG. NO. 3 DWG. NO. 3 NOR TH CAROL SEAL I5825 SEAL	DEPA FOR Of SR	RTMENT GENE BRIDG N SR 15 DE 1538 (SH	TE OF NORTH CA OF TRAN RALEIGH RAL DRA E OVER 46 (SHO BETWEEI AD END HADY GR	ROLINA NSPORTAT AWING HALL CF ALS ROA N AND ROVE CH.	TION REEK AD) RD.)
IIP	FILLO KLAPPETINI		REVIS	NONS	0.175	SHEET NO. S-3
350 609–3960 112	8/27/2015	<u>вт:</u> 1 2	DAIE:	NU. BY: 3 4	UAIE:	total sheets 17

										STRE	NGTH	I LIN	AIT SI	ΓΑΤΕ				SE	RVICE	III	LIMI	T STA	TE		
										MOMENT					SHEAR					•	MOMENT				
LEVEL		VEHICLE WEIGHT (W)	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	COMMENT NUMBER
		HL-93(Inv)	N/A		1.006		1.75	0.273	1.03	70′	EL	34.5	0.507	1.32	70'	EL	6.9	0.80	0.273	1.01	70′	EL	34.5		
DESTGN	Γ	HL-93(0pr)	N/A		1.341		1.35	0.273	1.34	70′	EL	34.5	0.507	1.72	70′	EL	6.9	N/A							
LOAD	Γ	HS-20(Inv)	36.000	2	1.306	47.02	1.75	0.273	1.34	70′	EL	34.5	0.507	1.65	70′	EL	6.9	0.80	0.273	1.31	70′	EL	34.5		
RATING	Γ	HS-20(0pr)	36.000		1.74	62.64	1.35	0.273	1.74	70′	EL	34.5	0.507	2.14	70′	EL	6.9	N/A							
	SNSH	13.500		2.917	39.379	1.4	0.273	3.75	70′	EL	34.5	0.507	4.87	70′	EL	6.9	0.80	0.273	2.92	70′	EL	34.5			
		SNGARBS2	20.000		2.187	43.741	1.4	0.273	2.81	70′	EL	34.5	0.507	3.47	70′	EL	6.9	0.80	0.273	2.19	70′	EL	34.5		
		SNAGRIS2	22.000		2.077	45.69	1.4	0.273	2.67	70′	EL	34.5	0.507	3.23	70′	EL	6.9	0.80	0.273	2.08	70′	EL	34.5		
		SNCOTTS3	27.250		1.452	39.565	1.4	0.273	1.87	70′	EL	34.5	0.507	2.43	70′	EL	6.9	0.80	0.273	1.45	70′	EL	34.5		
		SNAGGRS4	34.925		1.218	42.554	1.4	0.273	1.57	70′	EL	34.5	0.507	2.03	70'	EL	6.9	0.80	0.273	1.22	70′	EL	34.5		
		SNS5A	35.550		1.191	42.346	1.4	0.273	1.53	70′	EL	34.5	0.507	2.06	70′	EL	6.9	0.80	0.273	1.19	70′	EL	34.5		
		SNS6A	39.950		1.095	43.747	1.4	0.273	1.41	70′	EL	34.5	0.507	1.88	70′	EL	6.9	0.80	0.273	1.10	70′	EL	34.5		
I FGAI		SNS7B	42.000		1.043	43.801	1.4	0.273	1.34	70'	EL	34.5	0.507	1.85	70′	EL	6.9	0.80	0.273	1.04	70′	EL	34.5		
LOAD		TNAGRIT3	33.000		1.336	44.087	1.4	0.273	1.72	70'	EL	34.5	0.507	2.23	70′	EL	6.9	0.80	0.273	1.34	70′	EL	34.5		
RATING	[TNT4A	33.075		1.342	44.401	1.4	0.273	1.72	70'	EL	34.5	0.507	2.17	70′	EL	6.9	0.80	0.273	1.34	70′	EL	34.5		
	[TNT6A	41.600		1.1	45.746	1.4	0.273	1.41	70′	EL	34.5	0.507	1.98	70′	EL	6.9	0.80	0.273	1.10	70′	EL	34.5		
	ST	TNT7A	42.000		1.106	46.462	1.4	0.273	1.42	70′	EL	34.5	0.507	1.94	70′	EL	6.9	0.80	0.273	1.11	70'	EL	34.5		
	= [TNT7B	42.000		1.147	48.18	1.4	0.273	1.47	70′	EL	34.5	0.507	1.8	70′	EL	6.9	0.80	0.273	1.15	70′	EL	34.5		
		TNAGRIT4	43.000		1.089	46.838	1.4	0.273	1.4	70′	EL	34.5	0.507	1.74	70'	EL	6.9	0.80	0.273	1.09	70′	EL	34.5		
		TNAGT5A	45.000		1.026	46.175	1.4	0.273	1.32	70'	EL	34.5	0.507	1.74	70'	EL	6.9	0.80	0.273	1.03	70′	EL	34.5		
	[TNAGT5B	45.000	$\left\langle 3 \right\rangle$	1.013	45.579	1.4	0.273	1.3	70′	EL	34.5	0.507	1.66	70′	EL	6.9	0.80	0.273	1.01	70′	EL	34.5		



LRFR SUMMARY

ASSEMBLED BY : B. CHECKED BY : M.G.	N. GRADY CHEEK	Y DATE : 08/15 DATE : 08/15
DRAWN BY : CVC CHECKED BY : DNS	6/10 6/10	

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LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ_{DC}	γ_{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

NOTES:

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MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- 1. Ζ.
- 4
- (#) CONTROLLING LOAD RATING 1 DESIGN LOAD RATING (HL-93) 2 DESIGN LOAD RATING (HS-20) 3 LEGAL LOAD RATING ** ** SEE CHART FOR VEHICLE TYPE GIRDER LOCATION I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER





STD. NO. 24LRFR1_90S_70L



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	PROJEC	T NO.	BD-5	5111AB	
	Y	ADKI	N	CO	UNTY
	STATI	ON: 12	2+20.6	50 -L	
	SHEET 2 C	F 7			
	DEPA	STAT RTMENT	E OF NORTH CAR OF TRAN RALEIGH	NSPORTA	TION
unin LINA NA	F 24'-	2LAN -10″C 90	OF 7C LEAR)° SK	O'UNI ROAD EW	T WAY
	NO. BY:	REVIS	SIONS	DATE:	SHEET NO. S-6
(luek, Jr. 8/21/2015	1		3 4		total sheets 17
					-

STD. NO. 24PCS_27_90S_70L



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NOTES

------ ALUMINUM RAILS -------

MATERIAL FOR POSTS, BASES AND RAILS, EXPANSION BARS AND CLAMP BARS SHALL BE ASTM B221 ALLOY 6061-T6 MATERIAL FOR RIVETS SHALL BE ASTM B316 ALLOY 6061-T6.RIVETS SHALL BE STANDARD BUTTON HEAD AND CONE POINT COLD DRIVEN AS PER DRAWING.

THE BASE OF RAIL POSTS, OR ANY OTHER ALUMINUM SURFACE IN CONTACT WITH CONCRETE SHALL BE THOROUGHLY COATED WITH AN ALUMINUM IMPREGNATED CAULKING COMPOUND OF APPROVED QUALITY.

----- ANODIZING ------ALUMINUM FOR POSTS. BASES. RAILS. EXPANSION BARS. CLAMP BARS. RIVETS. CAPS. SHIMS. ATTACHMENT BRACKETS AND HOLD-DOWN PLATES SHALL BE ANODIZED BROWN.

ANY DAMAGE TO THE ANODIZED SURFACE OF THE RAIL OR COMPONENTS DURING CONSTRUCTION SHALL BE REPAIRED ACCORDING TO THE MANUFACTURER'S INSTRUCTIONS AT THE DIRECTION OF THE ENGINEER AND AT THE

THE CONTRACTOR SHALL SUBMIT A SAMPLE OF COMPATIBLE BROWN EXTERIOR ACRYLIC PAINT TO THE ENGINEER. THIS PAINT SHALL MATCH THE ANODIZED RAIL AS CLOSELY AS POSSIBLE. AFTER ERECTION OF THE ANODIZED ALUMINUM RAILING, ALL EXPOSED ANCHOR BOLTS, NUTS, WASHERS, MACHINE SCREWS, CAP SCREWS, BOLTS, ATTACHMENT BRACKETS, AND BUILT UP ANGLES SHALL BE COATED WITH TWO COATS OF THIS PAINT.

------ GENERAL NOTES -------

RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPLICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS. FOR END OF RAIL TO CLEAR FACE OF CONCRETE END POST DIMENSION, SEE SHEET 4 OF 7.

MATERIAL FOR ANCHOR STUDS SHALL BE ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. STUDS TO BE EMBEDDED 7" IN CONCRETE. NUTS SHALL BE AMERICAN STANDARD FINISHED HEXAGON THICK, CLASS 2B THREAD, AND MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL. WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL. ANCHOR ₱ SHALL BE AASHTO M270 GRADE 36.

CAP SCREWS SHALL BE ASTM F593 ALLOY 305 STAINLESS STEEL.

CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS AND POSTS. SHOP INSPECTION IS NOT REQUIRED. METAL RAIL POSTS SHALL BE SET NORMAL TO CURB GRADE.

METHOD OF MEASUREMENT FOR METAL RAILS: FOR LENGTH OF METAL RAILS TO BE PAID FOR, SEE THE STANDARD

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE THE CONTRACTOR MAY, AT HIS OPTION, HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT, THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER.

TO INSURE FUTURE IDENTIFICATION OF THE FABRICATOR, A PERMANENT IDENTIFYING MARK SHALL BE PLACED ON EACH POST. THE METHOD OF MARKING AND LOCATION SHALL BE SUCH THAT IT DOES NOT DETRACT FROM THE

SHIMS SHALL BE USED AS NECESSARY FOR POST ALIGNMENT.

ALLOY 6351-T5 MAY BE SUBSTITUTED FOR ALLOY 6061-T6 WHERE APPLICABLE.

MINOR VARIATIONS IN DETAILS OF METAL RAIL WILL BE CONSIDERED. DETAILS OF SUCH VARIATIONS, IF

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE $\frac{3}{4}$ " Ø BOLT IS 10 KIPS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS, NUTS AND WASHERS SHALL MEET THE SAME REQUIREMENTS AS THE ANCHOR STUDS. NUTS AND WASHERS FOR USE WITH THE ANCHOR ASSEMBLY.

NOTE : SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT. GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

> SEAL 20125

FOR ANODIZED ONE BAR METAL RAIL, SEE SPECIAL PROVISIONS

PAY LENGTH = 125.00 LIN.FT.





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NOTES
STRUCTURAL CONCRETE INSERT
T ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:
M STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND TH OF THREADS OF $1^{1}/_{2}^{\prime\prime}$.
WASHER.BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307.BOLT NIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER TE FOR THE $\frac{3}{4}$ " \varnothing X 15%" GALVANIZED BOLT AND WASHER.THEY SHALL

C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A $\frac{7}{16}$ " Ø WIRE STRUT WITH

NOTES

METAL RAIL TO END POST CONNECTION

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

A. $\frac{1}{2}$ " PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.

B. ¾" STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A ¾"Ø X 15%" BOLT WITH 2" O.D. WASHER IN PLACE. THE ¾"Ø X 15%" BOLT

C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.

E. $\frac{1}{2}$ " Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF ANODIZED ONE BAR METAL RAIL.

THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE $\frac{1}{2}$ " PLATES COMPLETE IN PLACE

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE $\frac{3}{4}$ " Ø X 15%" BOLT WITH WASHER SHALL BE REPLACED WITH A $\frac{3}{4}$ " Ø X 6 $\frac{1}{2}$ " BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE $\frac{3}{4}$ " Ø X 1 $\frac{5}{8}$ " BOLT SHALL APPLY TO THE $\frac{3}{4}$ " Ø X 6 $\frac{1}{2}$ " BOLT. FIELD TESTING OF THE

PROJECT NO. BD-5111AB									
YADKINCOUNTY									
STATION: 12+20.60 -L-									
SHEET 4 C	SHEET 4 OF 7								
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD									
F	RAIL POST SPACINGS								
END OF RAIL DETAILS									
	REVIS		S	DATE	SHEET NO.				
1 2	DATE:	™. 3 4	BT:	DATE:	TOTAL SHEETS 17				

SEAL 20125 **NCINE** -Docusigned by: Marshall G. Churk, Jr. /24/2015

STD. NO. BMR2





END VIEW

PARAPET AND END POST FOR ONE BAR RAIL

DRAWN BY :	M. POOLE	DATE	: 08/15
CHECKED BY :	B. N. GRADY	DATE	: 08/15

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ELEVATION



PARAPET & END POSTS							
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
₩ B25	36	# 5	STR	22'-11"	860		
* E1	40	# 7	STR	2'-9″	225		
* F1	16	# 6	STR	3′-5″	82		
* S13	158	# 5	2	3′-8″	604		
₩ FP0X	Y COATED						
REINFORCING STEEL LBS. 1771							
CLASS AA CONCRETE CU.YDS. 9.7							
TOTAL LIN.FT.OF 1'-O"X 1'-9 ¹ / ₂ " CONCRETE PARAPET 140.00					140.00		

	PROJE	CT NO. ADKI	BE N 12+2)-5111 CO	AB UNTY
		ON:	12 ' 2	0.00	
CAROLINA SSIONA EAL	DEP	ARTMENT	OF NORTH CAR OF TRAI RALEIGH	NSPORTA NSPORTA ARAPE DETA	TION
INEL		REVI	SIONS		SHEET NO.
5. Cheek, Jr.	NO. BY:	DATE:	NO. BY:	DATE:	S-9
⁵ 8/24/2015	1 2		3 4		TOTAL SHEETS 17

SEAL 20125

Marshall G. Une -6549D6EBAA3B405..



GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF GUARDRAIL ANCHOR AT END POST

ASSEMBLED BY : M. POOLE CHECKED BY : B. N. GRADY	DATE : 08/15 DATE : 08/15
DRAWN BY : MAA 5/10 CHECKED BY : GM 5/10	REV. 12/5/11 MAA/GM REV. 6/13 MAA/GM REV. 1/15 MAA/TMG

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SKE

NOTE	S
AIL ANCHOR ASSEMBLY SHALL CON BOLTS WITH NUTS AND WASHERS.	ISIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND
OWN PLATE SHALL CONFORM TO AA DN, THE HOLD-DOWN PLATE SHALL B FO M111.	ASHTO M270 GRADE 36.AFTER E HOT-DIP GALVANIZED IN ACCORDANCE
L CONFORM TO THE REQUIREMENTS THE REQUIREMENTS OF AASHTO ZED.AT THE CONTRACTOR'S OPTIO RS MAY BE USED AS AN ALTERNATE VASHERS.THEY SHALL CONFORM TO NTS OF ASTM A307.THE USE OF TH ER.	S OF ASTM A307 AND NUTS SHALL M291. BOLTS, NUTS AND WASHERS SHALL N, STAINLESS STEEL BOLTS, NUTS E FOR THE 1/8" Ø GALVANIZED BOLTS, OR EXCEED THE MECHANICAL HIS ALTERNATE SHALL BE APPROVED BY
RAIL ANCHOR ASSEMBLY IS REQUIF IS TO BE ATTACHED TO THE END F, SEE SKETCH.	RED AT ALL POINTS WHERE APPROACH OF THE PARAPET.FOR POINTS OF
ALLATION, THE EXPOSED THREAD C	OF THE BOLT SHALL BE BURRED WITH A
OF THE GUARDRAIL ANCHOR ASSEME IN PLACE, SHALL BE INCLUDED IN	BLIES WITH BOLTS, NUTS AND WASHERS THE VARIOUS PAY ITEMS.
CAL REINFORCING BARS MAY BE SH MBLY BOLTS.	IFTED SLIGHTLY IN THE END POST TO
HOLES SHALL BE FORMED OR DRIL BE PERMITTED. ANY CONCRETE DAM FISFACTION OF THE ENGINEER.	LED WITH A CORE BIT. IMPACT TOOLS IAGED BY THIS WORK SHALL BE REPAIRED
ACTOR SHALL SUBMIT A SAMPLE OF THE ENGINEER. THIS PAINT SHALL 'AS POSSIBLE. EXPOSED SURFACE TS,NUTS AND WASHERS SHALL BE COST OF THIS WORK SHALL BE I	F COMPATIBLE BROWN EXTERIOR ACRYLIC MATCH THE ANODIZED RAIL COLOR S OF THE HOLD-DOWN PLATE AND ALL COATED WITH TWO COATS OF THIS NCLUDED IN THE VARIOUS PAY ITEMS.
END OF UNIT - * * TCH SHOWING POI *LOCATION OF GUAN	The second secon
F	PROJECT NO. <u>BD-5111AB</u> <u>YADKIN</u> COUNTY STATION: <u>12+20.60</u> -L- SHEET 6 OF 7
Bocusigned by: Marshall G. Unit, Jr. 18/24/2015	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD GUARDRAIL ANCHORAGE DETAILS REVISIONS SHEET NO. BY: DATE: NO. BY: DETAILS
	(SHI Z) SID.NU.GKAJ

(SHT 2)



STRANDS				
	0.6″ØL.R.			
)	0.217			
GTH))	58,600			
SS))	43,950			

D	SLABS	S REQ	UIRED	
	NUMBER	LENGTH	TOTAL LENGT	Η
C.S.	2	70'-0″	140'-0"	
C.S.	7	70'-0″	490'-0"	
	9		630'-0"	

DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
SLAB UNIT	0.6″ØL.R. STRAND
ALONE IN PLACE)	2′′ 🕴
TO EAD LOAD **	^{''} ∕ı6'' ∳
	1 ⁵ ∕ı6'' ♦

BAR TYPES



BILL OF MATERIAL FOR ONE 70' CORED SLAB UNIT							
				EXTERI	OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B22	6	#4	STR	24'-6"	98	24'-6"	98
S10	8	# 5	3	4'-9"	40	4'-9"	40
S11	144	#4	3	5'-10″	561	5'-10"	561
* S12	79	# 5	1	5′-10″	481		
S14	4	#4	3	5′-7″	15	5'-7"	15
S15	4	# 5	3	7'-1"	30	7'-1"	30
REINFORCING STEEL LBS.		744		744			
* EPOXY COATED		Α	21				
REINFORCING STEEL LBS.		481					
7000 P.	S.I.CON	CRETE	CU. YDS.	. 11.8		11.8	
0.6″ØL	.R. STRA	NDS	No.	28		28	

GUTTERLINE ASPHA	LT THICKNESS & PARA	PET HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	PAPAPET HEIGHT @ MID-SPAN
70' UNITS	2 ³ / ₁₆ ''	1'-8 ³ / ₁₆ ''

CONCRETE RELE	ASE	STRENGTH
UNIT		PSI
70' UNITS		5500

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS.AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

ALL REINFORCING STEEL IN CONCRETE PARAPET SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

—DocuSigned by: Marshall G

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

	PROJECT Y STATION SHEET 7 OF	「 <u>NO.</u> 」 ADKI 1: <u>12</u> +	<u>BD</u> N -20.	<u>-5111</u> co 60 -	<u>AB</u> UNTY
CAROLINA SSIONA EAL DI25	DEPARTI PREST COI	STATE OF STAI STAI S'-O") RESS RED	F NORTH CAF TRAN ALEIGH NDAR X 2 ED (SLAB	ISPORTAT	TION RETE T
C. C. H. WINN		REVISIONS	5		SHEET NO.
Alunt No	NO. BY: [ATE: NO.	BY:	DATE:	S-11
هن رستین راند. همان (24/2015 میلیند) مرابع (24/2015 میلیند)	1	3 4			total sheets 17



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NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS. THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE CONCRETE PARAPET IS CAST IF SLIP FORMING IS USED. FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

TOP ELE	OF PILE VATIONS
1	784.98
2	784.68
3	784.38
4	784.08
5	783.78

	PROJEC	T NO	BD	-5111	AB
	Y <i>A</i>	DKI	N	CO	UNTY
	STATI	DN:	12+20	.60 -	-L <i>-</i>
	SHEET 1 OF	- 4			
<i>a.</i>	DEPA	STA RTMENT	TE OF NORTH CAR OF TRAI RALEIGH	OLINA NSPORTA	TION
		SUE	STRUCT	URE	
		END	BENT	No.1	
it in the second s					
ik, Jr.					
/21/2015		REVI	SIONS		SHEET NO.
	NO. BY:	DATE:	NO. BY:	DATE:	S-12
	1		3 4		TOTAL SHEETS 17
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NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS. THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE CONCRETE PARAPET IS CAST IF SLIP FORMING IS USED. FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

TOP ELE	OF PILE VATIONS
	786.08
2	785.78
3	785.48
4	785.18
5	784.88

PROJECT NO. B	D-5111AB
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COUNTY

YADKIN 12+20.60 -L-STATION:

SHEET 2 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE END BENT No.2



		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-13
1			3			TOTAL SHEETS
2			4			17

STD. NO. EB_27_90S



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STD. NO. EB_27_90S

8/21/2015



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		FOF	<u>r on</u>	<u>IE E</u>	ND BE	NT
4 ¹ / ₂ " 2'-5" 4 ¹ / ₂ "	BAR	NO.	SIZE	TYPE 1	LENGTH	WEIGHT
	B1 B2	16	#4	STR	17'-7"	188
НК. НК.	B3	9	#4	STR	2′-5″	15
1'-3'' LAP	D1	18	#6	STR	1'-6"	41
	H1	24	#4	2	('-10"	126
	К1	12	#4	STR	2'-11"	23
((5))	51	42	#4	3	7'-5"	208
	S2	42	#4	4	3'-2"	89
	S3	10	#4	5	6'-6"	43
<u>1'-8″∅</u>	V1	48	#4	STR	4'-8"	150
	REINF	ORCIN	NG STE ND BEN	EL IT)	1	835 LBS.
		A CO) NCRF T	F BRF4	KDOWN	
E OUT TO OUT.		FOR	ONE ENI	D BENT	[)	
P 12 X 53 STEEL PILES	POUR	#1 C	AP, LOW	IER PA	RT	10.5 C.Y.
LIN.FT.= 75		0	I WINC		JULLANS	
EXCAVATION	POUR	#2 U W	PPER P INGS	'ART O	F	2.0 C.Y.
OIL LIN.FT.= 15						
EXCAVATION	TOTAL	CLAS	SS A C	ONCRE	ΓE	12.5 C.Y.
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WINN TH CAROLINA			 k , 	K 1	4 •	\sim
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Marshall G. Cheek, Jr. 6549D6EBAA3B405... 8/21/2015

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SHEET NO. REVISIONS S-15 DATE: DATE: BY: BY: NO. TOTAL SHEETS 17

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RIP RAP AT END BENT No.2

ESTIMATED QUANTITIES					
BRIDGE @ STA.12+20.60 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE			
	TONS	SQUARE YARDS			
END BENT No.2	100	110			
TOTAL	100	110			

JND LINE	PROJEC	T NO. YADK DN: <u>1</u> 2	<u>B[</u> IN 2+2C	<u>)-5111</u> cc 0.60 -	<u>AB</u> DUNTY L-
SEAL 20125 CONSERVICE CONSTRUCTION CONSTRUCT	depa R]	rtment	E OF NORTH C OF TR RALEIGH	ANSPORTA	TION
-Docusigned by: Marshall G. Churk, Jr.		REVIS	SIONS		SHEET NO.
-6549D6EBAA3B405	NO. BY:	DATE:	NO. BY:	DATE:	5-16
8/21/2015	1		3 4		TOTAL SHEETS 17



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			BI	LL O	F MA	ATERIA	L
		APF	PROA	CH S	LAB	AT EB	No.1
LE, 4" Ø DRAINAGE PIPE,	k	BAR ¥ A1	NO. 13	SIZE #4	TYPE STR	LENGTH 25'-10"	WEIGHT 224
H THE STANDARD	E	A2	13	#4	STR	25'-10"	224
TION 1016.	*	₩ B1 B2	52 52	#5 #6	STR STR	<u>11'-2"</u> 11'-8"	606 911
NG FILL FACE OF OF APPROACH SLAB.	F	REINF	ORCIN	G STEE	L	LBS.	1135
WAY STANDARD DRAWINGS.	k	₩ EPOX REI	XY CO NFORC	ATED ING ST	EEL	LBS.	830
THE BRIDGE AND SHALL	С	CLASS	AA C	ONCRET	E	C. Y.	16.0
		APF	ROA	CH S	LAB	AT EB	No.2
	k l	BAR ⊯∆1	NO.	SIZE #4	TYPE STR	LENGTH	WEIGHT
		A2	13	#4	STR	25'-10"	224
	*	⊭ B1 B2	52 52	#5 #6	STR STR	11'-2" 11'-8"	606
				C STEE	1	<u></u> BS	1135
N Y WTTH	k		XY CO	ATED		LD3.	830
T MATERIAL							16.0
AIN		LASS	AA C	ONCRE I	Ł	C. Y.	16.0
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RGLASS ROVING T SOIL EROSION		S	PLI	CE L	ENG	THS	
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APPROACH SLAB.		1	¥4	2'-0	" 1'	-9″	
_		Ľ	*5	2'-6	<u>" 2'</u>	-2″	
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Marshall G. Unck, Jr. 6549D6EBAA3B405	NO. BY:	DATI		NO. BY:		DATE:	S-17
0, 21, 2015	1			3			TOTAL SHEETS

17

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS. CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES. ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

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DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS,AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" & STUDS BASED ON THE RATIO OF 3 - 7/8" & STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER

SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.





T .	RAFF1	s IC CC	TAT DI PL NT
RC THE F PROJE DATED CONST STE 11 11 11 12	DADWAY FOLLOWING ROADWAY SCT SERVICES UNIT O JANUARY 2013 ARE DERED A PART OF D. NO. 01.03 01.03 01.04 10.01 \$205.02 \$205.12	STANDARDS AS API STANDARDS AS API N.C. DEPARTMENT APPLICABLE TO THESE PLANS: <u>TITLE</u> MPORARY ROAD CLO MPORARY SHOULDED ATIONARY WORK ZO ARRICADES AVEMENT MARKINGS AVEMENT MARKINGS AVEMENT MARKINGS AVEMENT MARKINGS AVEMENT MARKINGS AVEMENT MARKINGS JARDRAIL & BARRI JARDRAIL & BARRI	DARD PEAR IN "R OF TRANSPO THIS PROJECT OSURES R CLOSURES R CLOSURES ONE SIGNS - LINE TY - 2 LANE - BRIDGES ER DELINEAT INEATION
FINAL	PAVEM	ENT MA	4 <i>RKI</i>
SYMBOL	DESCRIPTION	QUANTITY BREAKDOWN	PAY II
PA WHI PI YEL 2X = TWO A	PAVEMENT MARKING TE EDGELINE 2X LOW DOUBLE CENTER PPLICATIONS	LINES 2X 1692 LF	PAINT
	T T T T T T T T T T T T T T T T T T T	TRAFFF Image: Stress of the stress	

TE OF NORTH CAROLINA IVISION OF HIGHWAYS

AN FOR PROPOSED ROL, MARKING & DELINEATION

YADKIN COUNTY

DRAWINGS

ROADWAY STANDARD DRAWINGS"-PORTATION-RALEIGH, N.C., ECT AND BY REFERENCE HEREBY ARE

YPES & OFFSETS & MULTILANE ROADWAYS ATOR SPACING ATOR TYPES

ING SCHEDULE

TOTAL

QUANTITY

TEM _____

(4″)

TOTAL 3384 LF







GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

TRAFFIC PATTERN ALTERATIONS

A) NOTIFY THE ENGINEER TWENTY ONE (21) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATIONS.

SIGNING

1.

- B) PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.
- C) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT IN OPERATION.
- D) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

TRAFFIC CONTROL DEVICES

- E) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICENT LENGTH TO CLOSE ENTIRE ROADWAY.
- F) PLACE PAVEMENT MARKINGS ON THE FINAL SURFACE AS FOLLOWS: ROAD NAME MARKING

PAIN

- G) PLACE AT LEAST TWO APPLICATIONS OF PAINT PAVEMENT MARKINGS ON THE FINAL WEARING SURFACE ON NEW ASPHALT PAVEMENT. PLACE ADDITIONAL APPLICATIONS OF PAINT UPON SUFFICENT DRYING TIME, AS DETERMINED BY THE ENGINEER.
- H) CONTRACTOR TO MAINTAIN ACCESS TO ALL DRIVEWAYS, WITHIN THE PROJECT LIMITS AT ALL TIMES.

8/20/2015 R:\Traffi(mcole

STEP 1:	USING ROADWAY STANDARD DR
STEP 2:	DEMOLISH AND REMOVE EXIST
STEP 3:	COMPLETE CONSTRUCTION OF F (SEE ROADWAY PLANS & STRUC
STEP 4:	PLACE FINAL PAVEMENT MARK
STEP 5:	USING ROADWAY STANDARD DRA CONTROL DEVICES ERECTED IN



BD-SIIIAB	_
	ROADWAY STANDARDS AS APPEAR PROJECT SERVICES UNIT - N.C. DEPARTMENT OF DATED JANUARY 2012 ARE APPLICABLE TO THIS CONSIDERED A PART OF THESE PLANS: STD. NO. TITLE 1205.01 PAVEMENT MARKINGS - LIN 1205.12 PAVEMENT MARKINGS - TWO 1205.12 PAVEMENT MARKINGS - BRI 1261.01 GUARDRAIL AND BARRIER D 1262.01 GUARDRAIL END DELINEATION
THP PROJECT: PROJECT:	PAVEMENT MARKIN PA - 4" WHITE EDGELINE, PAIN PI - 4" DOUBLE YELLOW CENTER 2X = TWO APPLICATIONS

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

PAVEMENT MARKING PLAN YADKIN COUNTY

RD DRAWING

R IN "ROADWAY STANDARD DRAWINGS" -OF TRANSPORTATION - RALEIGH, N.C., S PROJECT AND BY REFERENCE HEREBY ARE

NE TYPES AND OFFSETS O-LANE AND MULTI-LANE ROADWAYS IDGES DELINEATOR SPACING DELINEATOR TYPES ION

SHE	EET	Ν

PMP-2

PMP-1

	THE FOLLOWING GENE THE CONSTRUCTION F OR DIRECTED BY THE
A)	INSTALL PAVEMENT MA AS FOLLOWS:
	ROAD NAME
	-L- LINE
B)	TIE PROPOSED PAVEME
C)	REMOVE/REPLACE ANY

NG SCHEDULE

INT, 2X ERLINE, PAINT, 2X

PAVEMENT MARKING PLANPLANS PREPARED BY :K. W. BISBY, P.E.PROJECT ENGINEERA. TUTTPROJECT DESIGNER

		PROJECT RE	FERENCE NO.	SHEET NO.
		BD-511	1AB	PMP-1
-		ł	SEAL 021047 NOTH CAROLA SEAL 021047	No Long
_		`		
	INDEX	<u>}</u>		
	DESCRIPTION			
	PAVEMENT MARK	ING PLAN TITLE	SHEET	
	PAVEMENT MARK	ING DETAIL		
)	

GENERAL NOTES

NERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF PROJECT, EXCEPT WHEN OTHERWISE NOTED IN THE PLAN, HE ENGINEER.

MARKINGS AND PAVEMENT MARKERS ON THE FINAL SURFACE

MARKING

PAINT

NONE

MARKER

MENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.

CONFLICTING/DAMAGED PAVEMENT MARKINGS AND MARKERS.



900 RIDGEFIELD DRIVE SUITE 350 RALEIGH, NORTH CAROLINA 27609–3960 NC LICENSE NO. F–0112 • (919) 878–9560





NOTE: PERIMETER EROSION CONTROL MEASURES SHALL BE INSTALLED DURING CLEARING AND GRUBBING PHASE.

=

NOTE: PLACE TEMPORARY ROCK SEDIMENT DAMS TYPE - B AND TEMPORARY ROCK SILT CHECKS TYPE - A AT DRAINAGE OUTLETS.

Description	Symbol	
Temporary Silt Fence	III III III	
Special Sediment Control Fence 🔼		
Temporary Berms and Slope Drains		
Silt Basin Type B		
Temporary Silt Ditch	TSD	
Temporary Diversion	——→ TD ——→	
Special Stilling Basin		
Rock Inlet Sediment Trap Type C	····	
Temporary Rock Silt Check Type-A		
Temporary Rock Silt Check Type=A v Matting and Polyacrylamide (PAM)	with	
Temporary Rock Silt Check Type-B		
Wattle	·····)	
Wattle with Polyacrylamide (PAM)	· · · · · · · · · · · · · · · · · · ·	
Temporary Rock Sediment Dam Type-B	B	
Rock Pipe Inlet Sediment Trap Type-A	······	PLANS PREPARED BY :
		RUMMEL, KLEPPER & KAHL, LLP
		900 RIDGEHELD DRIVE SUITE 350
		NC LICENSE NO. F-0112 • (919) 878-956

		<u>Sed.</u> #	D
		1605.01	Т
	AUDREY BURNETTE	1606.01	$\mathbb{S}_{\mathbb{I}}$
		1622.01	T
		1630.02	Si
		1630.03	\mathbb{T}
	3081	1630.05	\mathbb{T}
9019 CTANDADD SDECIEICATIC		1630.06	$\mathbb{S}_{\mathbb{I}}$
ZUIZ SIANDARD SPECIFICATIC	LEVEL IIIA CERTIFICATION NO.	1632.03	R
SAIS CTRANTINAT		1633.01	Т
ZUIZ SIAMDAI	KD DRAWINGS		T
1604.01 Railroad Frosion Control Detail	1632.01 Rock Inlet Sediment Tran Type A		\mathbb{N}
1605.01 Temporary Silt Fence	1632.02 Rock Inlet Sediment Trap Type B	1677.09	ግ
1606.01 Special Sediment Control Fence	1632.03 Rock Inlet Sediment Trap Type C	1033.02	Л
1607.01 Gravel Construction Entrance	1633.01 Temporary Rock Silt Check Type A		И
1622.01 Temporary Berms and Slope Drains	1633.02 Temporary Rock Silt Check Type B		
1030.01 Kiser Basin 1630.02 Silt Basin Type B	1634.01 Temporary Rock Sediment Dam Type A		И
1630.02 Sht bash Type D 1630.03 Temporary Silt Ditch	1634.02 Temporary Kock Sediment Dam Type B 1635.01 Rock Pine Inlet Sediment Tran Type A		
1630.04 Stilling Basin	1635.02 Rock Pipe Inlet Sediment Trap Type A	1634 09	ጥ
1630.05 Temporary Diversion	1640.01 Coir Fiber Baffle	1007.02	л
1630.06 Special Stilling Basin	1645.01 Temporary Stream Crossing	1635.01	R
1031.01 Matting Installation			~ `

Description Symbol
Temporary Silt Fence
Special Sediment Control Fence
Temporary Berms and Slope Drains
Silt Basin Type B·····
Temporary Silt Ditch
Temporary Diversion
Special Stilling Basin
Rock Inlet Sediment Trap Type C
Temporary Rock Silt Check Type=A
Temporary Rock Silt Check Type"A with Matting and Polyacrylamide (PAM)
Temporary Rock Silt Check Type-B
Wattle
Wattle with Polyacrylamide (PAM)
Temporary Rock Sediment Dam Type-B
Rock Pipe Inlet Sediment Trap Type-A

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TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)

See Inset A NOTES

USE EXCELSIOR FOR MATTING MATERIAL AND ANCHOR MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

INITIALLY APPLY 3.5 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.

PROJECT REFERENCE NO

BD-5IIIAB

SHEET NO. EC-2

WATTLE BARRIER DETAIL

NOTES:

USE MINIMUM 18 IN. NOMINAL DIAMETER EXCELSIOR WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 2 TO 3 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

CROSS SECTION.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 25 FT.

FILL SLOPE

INSET A

PROJECT REFERENCE NC	D. SHEET NO.
BD-5IIIAB	EC-2A
R/W SHEET N	٩٥.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL

TOP VIEW

SITE DESCRIPTION

PERIMETER DIKES, SWALES, DITCHES AND S

HIGH QUALITY WATER (HQW) ZONES

SLOPES STEEPER THAN 3:1

SLOPES 3:1 OR FLATTER

ALL OTHER AREAS WITH SLOPES FLATTER

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

	STABILIZATION TIME	7//
SLOPES	7 DAYS	NONE
	7 DAYS	NONE
	7 DAYS	IF SLOPES Not stee
	14 DAYS	7 DAYS F Length.
R THAN 4:1	14 DAYS	NONE, EXC

PROJECT REFERENCE NO.		SHEET NO.
BD-5IIIAB		EC-3
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER

MEFRAME EXCEPTIONS S ARE 10' OR LESS IN LENGTH AND ARE EPER THAN 2:1, 14 DAYS ARE ALLOWED. FOR SLOPES GREATER THAN 50' IN CEPT FOR PERIMETERS AND HQW ZONES.

MATTING FOR EROSION CONTROL

CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE	(SY)	CONST SHEET NO.	LINE	
4	- / -	10+25	10+75	RT	15				
4	- 🗸 -	+00	11+25	LT	10				
4	- / -	11+25	+59	LT	10				
			SUE	BTOTAL	35				
MISCELLANEC	DUS MATTING TO BE IN	ISTALLED AS DIRE	CTED BY THE	ENGINEER	15				AD
				TOTAL	50				
				SAY	50				

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION SUMMARY SHEET

PERMANENT SOIL REINFORCEMENT MAT

PROJECT REFERENCE NO). SHEET NO.
BD-5IIIAB	EC-3A
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

FROM STATION	TO STATION	SIDE	ESTIMATE (SY)
	SUE	BTOTAL	00
DITIONAL	PSRM TO BE I	NSTALLED	0
		TOTAL	00
		SAY	00
	1	1	

REFORESTATION

□ TREE REFORESTATION SHALL BE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

Rŀ _____

REFORESTATION		
MIXTURE, TYPE, SIZE, AND FURNISH SHALL C	ONFORM TO THE FOLI	LOWING:
25% LIRIODENDRON TULIPIFERA	TULIP POPLAR	12 in – 18 in BR
25% PLATANUS OCCIDENTALIS	SYCAMORE	12 in – 18 in BR
25% FRAXINUS PENNSYLVANICA	GREEN ASH	12 in – 18 in BR
25% BETULA NIGRA	RIVER BIRCH	12 in – 18 in BR

25%	BETULA	NIGRA	R	J

STATE	STATE PROJECT REFERENCE NO.			TOTAL SHEETS
N.C.	F	3D-5111AB	RF–1	
STATE PROJ. NO.		F. A. PROJ. NO.	DESCRIPT	ION

REFORESTATION DETAIL SHEET

N.C.D.O.T. - ROADSIDE ENVIRONMENTAL UNIT

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		0	2.5 5	P	ROJ. REFERENCE BD-5111	ce no. AB	sheet no. X-5
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							810
							805
							800
							795
							700
							/90
							785
							780
							876
							815
							810
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							790
							785
							780
40	45	50	55	60	65	/0	15 775
